

ModTruss 6" Steel Truss Load Table

With Splice Plates at connections



Span Feet (Meters)	Uniformly Distributed Load		Center Point Load		Third Point Load Total Load = Point Load x 2		Quarter Point Load Total Load = Point Load x 3		Fifth Point Load Total Load = Point Load x 4	
	Total Load Pounds (kgs)	Deflection Inches (mm)	Point Load Pounds (kgs)	Deflection Inches (mm)	Point Load Pounds (kgs)	Deflection Inches (mm)	Point Load Pounds (kgs)	Deflection Inches (mm)	Point Load Pounds (kgs)	Deflection Inches (mm)
5 (1.52)	8,906 (4,039.69)	0.09 (2.286)	4,432 (2,010.32)	0.07 (1.78)	3,334 (1,512.28)	0.09 (2.29)	2,223 (1,008.34)	0.08 (2.03)	1,854 (840.96)	0.09 (2.29)
10 (3.04)	4,389 (1,990.82)	0.37 (9.40)	2,194 (995.18)	0.29 (7.37)	1,635 (741.62)	0.37 (9.40)	1,090 (494.42)	0.35 (8.89)	911 (413.22)	0.37 (9.40)
15 (4.57)	2,854 (1,294.55)	0.83 (21.08)	1,427 (647.28)	0.67 (17.02)	1,070 (485.34)	0.85 (21.59)	714 (323.87)	0.80 (20.32)	595 (269.89)	0.84 (21.34)
20 (6.09)	2,065 (936.67)	1.48 (37.59)	1,033 (468.56)	1.21 (30.73)	774 (351.08)	1.51 (38.35)	516 (234.05)	1.43 (36.32)	430 (195.05)	1.49 (37.85)
25 (7.62)	1,575 (714.41)	2.32 (58.93)	787 (356.98)	1.91 (48.51)	591 (268.07)	2.36 (59.94)	394 (178.72)	2.24 (56.90)	328 (148.78)	2.34 (59.44)
30 (9.14)	1,234 (559.73)	3.34 (84.84)	617 (279.87)	2.79 (70.87)	463 (210.01)	3.40 (86.36)	308 (139.71)	3.23 (82.04)	257 (116.57)	3.36 (85.34)

ModTruss 6" Steel Truss Load Table

Without Splice Plates at connections



Span Feet (Meters)	Uniformly Distributed Load		Center Point Load		Third Point Load Total Load = Point Load x 2		Quarter Point Load Total Load = Point Load x 3		Fifth Point Load Total Load = Point Load x 4	
	Total Load Pounds (kgs)	Deflection Inches (mm)	Point Load Pounds (kgs)	Deflection Inches (mm)	Point Load Pounds (kgs)	Deflection Inches (mm)	Point Load Pounds (kgs)	Deflection Inches (mm)	Point Load Pounds (kgs)	Deflection Inches (mm)
5 (1.52)	10,025 (4,547.26)	0.09 (2.28)	4,469 (2,027.10)	0.09 (2.29)	3,118 (1,414.30)	0.08 (2.03)	2,113 (958.44)	0.09 (2.29)	1,603 (727.10)	0.10 (2.54)
10 (3.04)	5,550 (2,517.43)	0.32 (8.13)	2,350 (1,065.94)	0.28 (7.11)	1,619 (734.36)	0.26 (6.60)	1,214 (550.66)	0.32 (8.13)	982 (445.42)	0.32 (8.13)
15 (4.57)	1,569 (711.68)	0.95 (24.13)	1,244 (564.26)	1.60 (40.64)	836 (379.20)	1.73 (43.94)	584 (264.89)	1.67 (42.42)	486 (220.44)	1.77 (44.96)
20 (6.09)	1,194 (541.58)	1.06 (26.92)	822 (372.85)	1.04 (26.42)	740 (335.65)	1.05 (26.67)	576 (261.26)	1.02 (25.91)	466 (211.37)	1.02 (25.91)
25 (7.62)	1,139 (516.64)	1.39 (35.31)	699 (317.06)	1.37 (34.80)	498 (225.88)	1.49 (37.85)	349 (158.30)	1.55 (39.37)	278 (126.09)	1.53 (38.86)
30 (9.14)	1,164 (527.98)	2.22 (56.18)	639 (289.84)	2.40 (60.96)	433 (196.40)	2.38 (60.45)	306 (138.79)	2.49 (63.25)	255 (115.66)	2.61 (66.29)

Information extracted from the structural report by Clark Reder Engineering | 10091 Mosteller Lane | West Chester OH 45069 | Ph 513-851-1223 | Date: 11/2/2018 | CRE Project No. 17.419.07 | Drawn by: JMR/DDL | S1.2

6" Steel Truss (unbraced length) Column Load Capacity	
10' (3.04 meters)	28,080 lbs (12,736.87 kg)
20' (6.09 meters)	20,430 lbs (9,266.89 kg)
30' (9.14 meters)	11,970 lbs (5,429.50 kg)

All columns are assumed to be pinned top and bottom and use an Effective Length Factor of K=1.0.

All capacities assume that no other shear, flexure, or torsional forces are applied to the column.

Information extracted from the structural report by Clark Reder Engineering
Date: 02/22/2019 | CRE Project No. 19.419.05 | Engineer: DJP